Groundwater Remedial Investigation Report, Final Garfield Avenue Group of Sites PPG, Jersey City, New Jersey

Appendix E

Implications of Soil Heterogeneities on Fate and Transport



# APPENDIX E – Implication of Soil Heterogeneities on Fate and Transport

- I. Relationship Between Soil Heterogeneities and Fate and Transport
- II. Matrix Diffusion Modeling Cr<sup>+6</sup> Distribution and Future Transport



#### I. Relationship Between Soil Heterogeneities and Fate and Transport

During implementation of the Phase II and Phase III IRM remediation and monitoring well networks, approximately 2,824 vertical feet of HPT boring was conducted at 54 different locations. Vertical depths ranged from approximately 22.08 feet (ft) below ground surface (bgs) to 85.05 feet bgs (-9.4 ft NAVD88 to -73.95 ft NAVD88). These borings were performed to obtain high-resolution hydraulic conductivity data within the Phase II and III IRM areas. This high-resolution IRM characterization information improved the understanding of site stratigraphic resolution and heterogeneities in the intermediate and deep water-bearing zones. The hydraulic conductivity response measured via HPT is obtained via minor injections of water from the tooling along the vertical extent of the boring, with concurrent measurements of downhole pressure change. This methodology has sufficient resolution to reliably identify hydraulic conductivity differences throughout the vertical extent of each boring. Outputs from the Phase II and III IRM HPT borings are included on **Figures E1** through **E33** and **Figures E34** through **E54**, respectively.

The data from individual high-resolution HPT borings are useful in understanding changes in relative permeability and apparent differences in boring-specific groundwater and Cr flux. A review of these individual borings also provides a clear indication that the migration of groundwater and Cr at Site 114 is not uniform. To support this analysis, the direct hydraulic conductivity data obtained from each HPT profile were sorted using a cumulative distribution function ( $CDF_K$ ) plot to group individual aquifer fractions for relative permeability evaluation (see inset

example at right) (Horst et al. 2017<sup>1</sup>). The CDF<sub>K</sub> figures are an effective method to sort and understand the relative fraction of hydraulic conductivity distribution on the yaxis (0.0 to 1.0) with the corresponding boring thickness along the x-axis. This improves visualization of the order of magnitude differences in hydraulic conductivity data to identify the fraction of the groundwater flow (approximated from the hydraulic conductivity) that occurs across the boring thickness. Based on the relative hydraulic conductivities observed across each HPT profile, the order of magnitude differences in relative permeability are illustrated by sorting them from most permeable to least permeable into three compartments: C1, C2, and C3. Given the order of magnitude changes between each compartment, these

Cumulative distribution function plot for 114-P2-IRM-32D



compartments identify the overall fraction of the soil profile through which 90%, 9%, and 1%, respectively, of the groundwater flux moves through Site 114 soils. In the inset figure at right for boring 114-P2-IRM-32D, the C1 compartment representing 90% of the flux (CDF<sub>K</sub> between 0.0 - 0.9) is encountered across

<sup>&</sup>lt;sup>1</sup> Horst, J., S. Potter, M. Schnobrich, N. Welty, A. Gupta, and J. Quinan. 2017. Advancing Contaminant Mass Flux Analysis to Focus Remediation: The Three-Compartment Model. Groundwater Monitoring & Remediation 37, no. 4, Fall.



approximately 31 feet of the boring, the C2 compartment representing 9% of the flux (CDF<sub>K</sub> between 0.90 – 0.99) is encountered across 7 feet of the boring, and the C3 compartment representing 1% of the flow (CDF<sub>K</sub> between 0.99 - 1.0) is encountered across 14 feet of the boring.

C1 and C3 are separated by two orders of magnitude difference in relative permeability, which defines the difference between advection (C1) and diffusion (C3) transport processes (ESTCP 2012<sup>2</sup>). C2 resides between these two zones and is dominated by slow advection processes. Both advection-dominated zones (C1 and C2) can be considered permeable, whereas the diffusion-dominated zone (C3) can be considered to have little to no permeability. The net result for the inset example on the previous page is that 38 feet of the boring (73%) consists of permeable soils and 14 feet of the boring (27%) consists of soils with little to no permeability.

A summary of the CDF<sub>K</sub> plot output metrics for each of the Phase II and III IRM borings are presented in **Tables E1** and **E2**. When the individual borings are sorted via the above methods, the total 1,560 feet of Phase II IRM HPT borings indicate that approximately 852 feet (55%) are identified as C1 soils, 306 feet (20%) are identified as C2 soils, and 401 feet (25%) are identified as C3 soils (**Table E1**). Similarly, the total 1,264 feet of Phase III IRM HPT borings indicate that approximately 693 feet (55%) are identified as C1 soils, 203 feet (16%) are identified as C2 soils, and 368 feet (29%) are identified as C3 soils (**Table E2**). The compiled totals for each zone in the Phase II and Phase III IRM borings are considered equivalent considering the heterogeneities of the GAG site soils. Combining all 2,824 feet of HPT boring information from both Phase II and III IRM areas, approximately 73% of the intermediate and deep waterbearing zones soils are considered permeable and 27% of these soils are characterized by little to no permeability.

<sup>&</sup>lt;sup>2</sup> ESTCP, 2012. Farhat, S.K., C.J. Newell, T.C. Sale, D.S. Dandy, J.J. Wahlberg, M.A. Seyedabbasi, J.M. McDade, and N.T. Mahler. *Matrix Diffusion Toolkit, developed for the Environmental Security Technology Certification Program (ESTCP) by GSI Environmental Inc., Houston, Texas.* 2012.

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# Matrix Diffusion Modeling – Cr<sup>+6</sup> Distribution and Future Transport

To understand the effects of diffusive Cr<sup>+6</sup> mass discharge from the low permeability strata of the intermediate and deep water-bearing zones, two different matrix diffusion modeling methods were used to simulate both the loading and release phases between higher permeability and adjacent low permeability soils. These include a Numerical Diffusion Model developed by Arcadis as well as the Matrix Diffusion Toolkit (ESTCP 2012<sup>3</sup>). These models were designed to evaluate matrix diffusion relationships between soils of varied relative permeability. Both models leverage a loading phase wherein a groundwater analyte migrates via diffusion from a higher permeability advection zone into a lower permeability diffusion zone. The loading phase is then followed by a subsequent diffusion phase to assess: 1) mass discharge from the low permeability to the high permeability zone (i.e., back or reverse diffusion); and 2) continued diffusion deeper into the low permeability zone (i.e., forward diffusion).

Both models were set up using a range of parameters applicable to understanding diffusion at and near Site 114. The advective zone was assumed to be 10 feet (ft) thick with an average groundwater flow velocity of 0.01 ft/day. Groundwater velocity was determined within the sheet pile area using a hydraulic conductivity value of 5 ft per day, a gradient of 0.00025 ft per ft, and a mobile porosity of 12.5%. This slow flow rate is considered representative of flow within the sheet pile, but groundwater velocity does not have a significant bearing on the diffusion model outputs. The underlying mass storage zone was represented by a 10 ft thick clay unit with a total porosity (n) of 40% and no advective flow (Text Figure 1). While many of the clay sequences identified in the intermediate and deep waterbearing-zones are thinner than 10 ft, this thickness was selected to understand the extent of Cr<sup>+6</sup> diffusion over the modeled durations knowing that the



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outputs can therefore be applied to any other clay sequence of comparable or thinner thickness. An effective diffusivity ( $D_e$ ) term of 5.60E-6 cm<sup>2</sup>/sec was assigned for Cr<sup>+6</sup> based on the diffusivity (D) term of chloride in water value of 1.89E-5 cm<sup>2</sup>/sec (Cussler, 1984<sup>4</sup>) using the equation  $D_e = D^*n^{1.33}$ .

To simulate the forward diffusion of  $Cr^{+6}$  from the advective zone to the mass storage zone, a fixed concentration of 100 milligrams per liter (mg/L)  $Cr^{+6}$  was simulated in the advective zone for a period of 50 years. The 100 mg/L  $Cr^{+6}$  concentration was selected as it applies to most of the impacted groundwater area at Site 114. Investigation data have demonstrated that  $Cr^{+6}$  concentrations are higher

<sup>&</sup>lt;sup>3</sup> ESTCP, 2012. Farhat, S.K., C.J. Newell, T.C. Sale, D.S. Dandy, J.J. Wahlberg, M.A. Seyedabbasi, J.M. McDade, and N.T. Mahler, 2012. Matrix Diffusion Toolkit, developed for the Environmental Security Technology Certification Program (ESTCP) by GSI Environmental Inc., Houston, Texas.

<sup>&</sup>lt;sup>4</sup> Cussler, E.L. 1984. <u>Diffusion: Mass transfer in fluid systems</u>. Cambridge University Press.



than this in select areas of the site and have also demonstrated that Cr<sup>+6</sup> attenuates in concentration with increased depth in other areas. Ultimately, the resulting outputs of the matrix diffusion modeling programs are proportional to the starting concentration, so can be easily modified based on the assumed starting concentration relative to that applied for the model. As an example, if 200 mg/L was used as the loading concentration, the model outputs would be two-fold higher than the concentration obtained using a 100 mg/L loading value. Given the spatial and vertical variability of Cr<sup>+6</sup> at the site, the 100 mg/L number therefore serves as a representative midpoint of what is found at Site 114 and is easily scalable to guide interpretations. While chromite ore processing predates the assumed timeframe of the 50-year loading period, it is challenging to pinpoint exactly when dissolved Cr<sup>+6</sup> migrated through the shallow zone, the meadow mat, and then the more permeable soils of the intermediate and deep water-bearing-zones where it began diffusing into lower permeability soils. Based on plume dynamics, it is expected that Cr<sup>+6</sup> transport would have started at lower concentrations and then built in Cr<sup>+6</sup> concentration over time. Given that the 50-year modeling duration assumes a stable Cr<sup>+6</sup> concentration (100 mg/L) and that diffusion is concentration dependent, the 50-year duration was chosen to account for the dynamics of early Cr<sup>+6</sup> plume invasion and to allow a long enough timeframe to understand matrix diffusion effects. Considering that the outputs of both models demonstrate that 50 years is sufficient longevity for Cr<sup>+6</sup> diffusion through the 10-ft clay zone, variability around the 50-year timeframe does not substantively affect the analysis.

The final modeling step included a step governed by diffusion only. After the loading period of 50 years, the advective zone  $Cr^{+6}$  source concentration in both models was set to 0 mg/L to allow projection of the future mass discharge of  $Cr^{+6}$  from the low permeability soils back into the advective zone. This reverse modeling portion was run for an additional 50 years. The outputs of both models employed are outlined in the sections below.

#### **Numerical Modeling Approach**

For the numerical modeling simulation, the model domain established as outlined on the previous page and used to simulate 100 years of diffusion split between both the forward and reverse diffusion stages. The model was constructed in accordance with Text Figure 1 and with the model parameters stated on the previous page to allow evaluation of differences in diffusion profiles along a plume flow path (locations B and C) downgradient of the initial source (location A).

The results profiles for locations B and C are plotted on the following page. Solid lines represent incremental 5-year intervals during the forward diffusion phase and dashed lines represent the projections once the  $Cr^{+6}$  concentration in the permeable zone is reduced to 0 mg/L after year 50.

The profiles on Text Figures 2 and 3 show that during the 50-year loading period,  $Cr^{+6}$  concentrations are expected to gradually diffuse into at least 10 feet of lower permeability soils where they decline in concentration with depth. Following reduction of the  $Cr^{+6}$  concentration to 0 mg/L after year 50, the reverse diffusion period begins and lower concentrations of  $Cr^{+6}$  are predicted within the advective zone just above the interface. The highest mass discharge from the low permeability zone into the advective zone occurs immediately following source removal (years 51 through 55), following which mass discharge release from the low permeability soils declines. This decrease in mass discharge occurs due to the reduced concentrations of  $Cr^{+6}$  at the diffusion zone interface associated with the loss of  $Cr^{+6}$  from the low to high permeability zones and the further diffusion of  $Cr^{+6}$  deeper into the low permeability zone. This pattern progresses with time, with sequential declines in mass discharge from low to high permeability zone where it stabilizes. While modeled for a 10-foot sequence of low permeability zone soils, this same relative pattern would be expected in zones that are both narrower or thicker in vertical thickness.





Text Figure 2 – Forward and reverse diffusion projections for 100-year model duration 5 ft downgradient from source (B).



Text Figure 3 – Forward and reverse diffusion projections for 100-year model duration 10 ft downgradient from source (C).



The mass discharge profiles modeled on the previous page can be leveraged to calculate the expected Cr<sup>+6</sup> concentration in advection zone groundwater for each of the 5-year time periods between year 50 and 100. The graph presented on Text Figure 4 includes the predicted concentration in a theoretical monitoring well installed at both locations B and C and screened across 10 ft of the advection zone.



Text Figure 4 – Predicted  $Cr^{+6}$  concentrations at two theoretical monitoring wells installed at locations 5 ft (B) and 10 ft (C) within the model domain.

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# **Matrix Diffusion Toolkit Application**

The second model used to assess both forward and reverse Cr<sup>+6</sup> diffusion was the Matrix Diffusion Toolkit, which is an industry-accepted and publicly available method served to validate the simulations of the first modeling approach. The estimates were developed using the square root model input parameters, the same model domain size specifications, and the same source Cr<sup>+6</sup> concentration as outlined in the preceding section. The modeling process adhered to the guidance provided and used the Dandy-Sale Model (DSM) approach. The conceptual modeling approach related to both source loading and diffusion phases is outlined on Text Figure 5 at right.

A summary of the model data input parameters used as a basis for diffusion simulation are included in Text Figure 6.



Text Figure 5 - Dandy-Sale Model Domain

These utilized the same value for molecular diffusion, assumed no sorption, and assumed a source concentration of 100 mg/L.

DSM Data Input Screen Matrix Diffusion Toolkit		Ve	rsion 1.23	TA INPUT INSTRUCTIONS Enter value directly.	
Site Location and ID: Garfield Ave				Value calculated by Toolkit. Do not enter data.	
1. SYSTEM UNITS			DN	APL Transmissive Zo	ne
2. HYDROGEOLOGY				GW Flow	
Transmissive Zone Description Transmissive Zone Effective Porosity	Sand	Sand	"4	"is only used to	4
Low-k Zone Description	Clay	Clay	de	Incentration for the recentration for the relical plane source.	
Low-k Zone Total Porosity	n' 0.40	) (-)		Results	
Transmissive Zone Seepage Velocity	V 3.53E-06	(cm/sec)  Calculate V ?		Calculated Here	
3. TRANSPORT					
Key Constituent (enter directly or choose from drop down list) Plume Loading Concentration Immediately Above Low-k	Cr6			A A A	
Zone in Vertical Plane Source During Loading Period	C. 100	(mg/L) 💌		at .	
Molecular Diffusion Coefficient in Free Water	Do 1.89E-05	(om2/sec)			
Transmissive Zone Apparent Tortuosity Factor Exponent	P 0.33	3 (-)			
Low-k Zone Apparent Tortuosity Factor Exponent	p' 1.33	(-)	4. SOUR	CE ZONE CHARACTERISTICS	
Bulk Density of Transmissive Zone	ρ <sub>b</sub> 1.73	3 (g/mL)	Sour	ce Zone Length	L 3.05 (m)
Bulk Density of Low-k Zone	ρ' <sub>δ</sub> 1.59	) (g/mL)	Sour	ce Zone Width	W 1 (m)
Distribution Coefficient or Transmissive Zone Fraction of Organic Carbon	K <sub>d</sub> f <sub>oc</sub> 0.00E+00	(mL/g) Calculated R (-) 1.00	Tran: Sour Sour 5. GENE See I	sverse (Vertical) Hydrodynamic Dispersivity ce Loading Starts in Year ce Removed in Year RAL Release Period Results for:	a; <u>1.00E-03</u> (m) <u>Restore</u> ? 1970 (format: yyyy) 2020 (format: yyyy)
Organic Carbon Partitioning Coefficient	K 0.00E+00	(1/8/2)	v	ear	2025 (format www)
	N.60 0.002.00			ateral Distance from Source	3 (m)
			-	enth into Low-k Zone	7 305 (m)
10					2 0.00 0.00
Next St	ten:	New Site/Clear Data	Paste Example		
Show G	raph	Save Data	Load Data	HELP	
Show Previou	s Results	Return to Model Selection Screen	Return to Main S	creen	

Text Figure 6 - Matrix Diffusion Toolkit assumptions and input parameters

Similar to the outputs of the first numerical model, the DSM methodology allows estimation of the potential diffusion depths of  $Cr^{+6}$  during the initial 50-year loading period and then allows time-based estimates of the advective zone concentrations following source removal. Using the input entries from the



DSM interface on the preceding page, the plots for  $Cr^{+6}$  diffusion into the low permeability zone and an example profile for  $Cr^{+6}$  concentrations within the advective zone are included on Text Figure 7.



Text Figure 7 - Projected Cr<sup>+6</sup> concentrations in low permeability soils during year 55, 5 years following source elimination.

The Matrix Diffusion Toolkit approach does not allow outputs of multiple time series data over a prolonged timeframe similar to Text Figure 4; therefore, the data from the individual time periods (e.g., years 55, 60, 65, etc) were compiled from the 50- to 100-year durations to develop profiles for comparison both modeling approaches. Text Figure 8 is therefore provided the compare the predicted Cr<sup>+6</sup> concentrations within the advection zone in a theoretical 10-ft monitoring well for both modeling approaches.



Text Figure 8 - Predicted Cr<sup>+6</sup> concentrations at theoretical monitoring wells within modeled domains.



### **Diffusion Model Summary**

Separate modeling runs were conducted in two different modeling packages to assess consistency of the results. The concentrations of  $Cr^{+6}$  predicted in monitoring wells installed at the diffusion interface from both models are in good general agreement. Both models predicted that the highest  $Cr^{+6}$  rate of diffusion-based  $Cr^{+6}$  discharge occurs within the first 5 to 10 years after removing the  $Cr^{+6}$  model source (55 to 60 years from beginning of loading), following which they gradually decline to less than 1 mg/L by year 15 (65 years from beginning of loading) (Text Figure 8).

The diffusion modeling provides information useful to understanding the relative distribution of  $Cr^{+6}$  within lower permeability soils and its expected behavior following source removal in the more permeable zone soils. These are conceptualized in Section I as the C1 and C2 compartments which are dominated by advection, and the C3 compartment which is dominated by diffusion. In conjunction with the HPT  $CDF_{K}$ analysis from Site 114, these collective findings have the following implications for the Conceptual Site Model and remedy development:

- Low permeability materials account for approximately 27% of the intermediate and deep waterbearing zones and are expected to contain Cr<sup>+6</sup> concentrations that are within one order of magnitude less than the concentrations measured in the adjacent higher permeability zones (prior to any treatment). The Cr<sup>+6</sup> concentrations within these low permeability soils decline with increased vertical thickness, and diffusion gradients limit migration deeper into the soil column.
- The models predicted that dissolved Cr<sup>+6</sup> is expected to reside in the low permeability soils for an extended period (at least an additional 50 years), but also demonstrate that the slow rate of diffusion serves to immobilize Cr<sup>+6</sup> within these soils, limits future migration of Cr<sup>+6</sup>, and stabilizes Cr<sup>+6</sup> within the aquifer.
- The expected release of Cr<sup>+6</sup> from the low permeability zone will be highest within the first several years following Cr<sup>+6</sup> treatment, following which mass discharge from low to high permeability zones will decline significantly within 5 to 10 years.



#### Attachments

Table E1	Phase II IRM CDF $\kappa$ Summary Metrics
Table E2	Phase III IRM CDF $\kappa$ Summary Metrics
Table E2Figure E1Figure E2Figure E3Figure E4Figure E5Figure E6Figure E7Figure E8Figure E10Figure E11Figure E12Figure E13Figure E14Figure E15Figure E16Figure E17Figure E18Figure E19Figure E19Figure E20Figure E21Figure E22Figure E23Figure E24Figure E25	Phase III IRM CDF <sub>K</sub> Summary Metrics 114-P2-MW-1D 114-P2-MW-2D 114-P2-MW-3D 114-P2-MW-5D 114-P2-MW-6D 114-P2-MW-6D 114-P2-MW-7D 114-P2-MW-9D 114-P2-MW-9D 114-P2-MW-10I 114-P2-MW-10I 114-P2-MW-11I 114-P2-MW-12D 114-P2-IRM-11D 114-P2-IRM-15D 114-P2-IRM-15D 114-P2-IRM-15D 114-P2-IRM-35I 114-P2-IRM-35I 114-P2-IRM-35I 114-P2-IRM-53I 114-P2-IRM-55I 114-P2-IRM-69I 114-P2-IRM-74I
Figure E26	114-P2-IRM-76D 114-P2-IRM-87D
Figure E28	114-P2-IRM-89D
Figure E29	114-P2-IRM-92DR
Figure E30	114-P2-IRM-94I
Figure E31	114-P2-IRM-96I
Figure E32	114-P2-IRM-97I
Figure E33	114-P2-IRM-92D-2

P3-IRIVI-VAPUT
P3-IRM-VAP02
P3-IRM-VAP03
P3-IRM-VAP04
P3-IRM-VAP05
P3-IRM-VAP06
P3-IRM-VAP07
P3-IRM-VAP08
P3-IRM-VAP09
P3-IRM-VAP10
P3-IRM-VAP11
P3-IRM-VAP12
P3-IRM-VAP13
P3-IRM-VAP14
P3-IRM-VAP15
P3-IRM-VAP16
P3-IRM-VAP17
P3-IRM-VAP18
P3-IRM-VAP19
199-SS1
199-SS2

	Zone of Relative Permeability		
Phase II IRM Well ID	C1	C2	C3
MW-1D Thickness (	22.7	13.9	11.3
% of Bori	ng 47%	29%	24%
MW-2D Thickness (	t) 29.6	7.8	7.0
% of Bori	ng 67%	18%	16%
MW-3D Inickness (	T) 33.8	8.1	6.8
	18 09% (+) 18 3	11 /	14%
MW-4D % of Bori	40%	25%	35%
Thickness (	t) 27.2	10.1	13.3
MW-5D % of Bori	ng 54%	20%	26%
MW-6D Thickness (	t) 39.1	10.0	5.2
% of Bori	ng 72%	18%	10%
MW-7D Thickness (	t) 34.4	9.1	8.8
% of Bori	ng 66%	17%	1/%
MW-8I % of Bori	() 28.0	11.9	24.3
Thickness	t) 31.0	8.9	9.6
MW-9D % of Bori	ng 63%	18%	19%
Thickness (	t) 30.2	7.1	14.0
% of Bori	ng 59%	14%	27%
MW-11I Thickness (	40.8	9.4	13.5
% of Bori	64%	15%	21%
MW-12D Northeast MW-12D	t) 29.7	6.3	15.1
% of Bori	ng 22.0	83	13.3
IRM-1I Thickness (	t) 50%	19%	30%
% of Bori	ng 29.4	12.0	7.7
Thickness (	t) 60%	24%	16%
IBM-11D % of Bori	ng 27.5	10.0	9.4
Thickness (	t) 59%	21%	20%
IRM-15D % of Bori	ng 30.7	12.7	15.6
	T) 52%	9.7	20%
IRM-17D Thickness (	t) 76%	20%	4%
% of Bori	ng 28.8	8.3	13.8
IRM-29D Thickness (	57%	16%	27%
IBM-32D % of Bori	ng 31.4	6.5	14.1
Thickness (	t) 60%	13%	27%
IRM-35I % of Bori	ng 8.9	1.9	12.1
Inickness (	t) <u>39%</u>	8%	53%
IRM-42D Thickness (	18 31.1 (+) 58%	21%	21%
% of Bori	ng 22.3	13.5	11.3
IRM-53I Thickness (	(t) 47%	29%	24%
IRM-551 % of Bori	ng 29.9	6.9	14.5
Thickness (	t) 58%	13%	28%
IRM-69I % of Bori	ng 20.5	7.7	8.0
% of Bori	18 5/% (t) 21 0	<u>21%</u> 12.2	22% 15 Q
IRM-74I % of Bori	43%	26%	31%
Thickness (	t) 17.1	8.5	10.6
IRM-76D % of Bori	ng 47%	23%	29%
IRM-87D Thickness (	t) 21.1	9.6	14.3
% of Bori	ng 47%	21%	32%
IRM-92DR Thickness (	t) 14.1	6.5	3.8
% of Bori	ng 58%	27%	16%
IRM-94I A of Bori	ין בא.ש ועם 58%	21%	21%
Thickness (	tt) 25.8	6.5	19.1
IRM-96I % of Bori	ng 50%	13%	37%
IRM-97I Thickness (	t) 28.5	5.8	16.8
% of Bori	ng 56%	11%	33%
Thickness (	(t) 15.1	9.4	17.0
% of Bori	ng 36%	23%	41%
IRM-92D-2	u 18.7	5.3 21%	1./
% OI BOII Thickness (	t) 851.9	306.5	401.7
Combined % of Bori	ng <b>54.6%</b>	19.6%	25.7%

	Zone of Relative Permeability		
Phase III IRM VAP ID	C1	C2	C3
Thickness (ft)	24.3	5.9	5.1
P3-IRWI-VAPU1 % of Boring	69%	17%	14%
Thickness (ft)	27.6	11.0	13.0
P3-IRM-VAP02 % of Boring	53%	21%	25%
Thickness (ft)	26.3	10.1	23.1
P3-IRM-VAP03 % of Boring	44%	17%	39%
Thickness (ft)	42.1	9.7	19.7
P3-IRM-VAP04 % of Boring	59%	14%	28%
Thickness (ft)	41.2	10.2	21.6
P3-IRM-VAP05 % of Boring	56%	14%	30%
Thickness (ft)	44.3	13.1	4.4
P3-IRM-VAP06 % of Boring	72%	21%	7%
Thickness (ft)	34.7	9.2	17.4
P3-IRM-VAP07 % of Boring	57%	15%	28%
Thickness (ft)	36.7	9.0	25.5
P3-IRM-VAP08 % of Boring	51%	13%	36%
Thickness (ft)	26.9	9.5	29.1
P3-IRM-VAP09 % of Boring	41%	14%	44%
Thickness (ft)	34.3	9.4	22.4
P3-IRM-VAP10 % of Boring	52%	14%	34%
Thickness (ft)	38.9	16.7	26.8
P3-IRM-VAP11 % of Boring	47%	20%	33%
Thickness (ft)	43.9	11.3	29.8
P3-IRM-VAP12 % of Boring	52%	13%	35%
% of Boring	29.2	8.5	31.3
P3-IRIVI-VAP13 Thickness (ft)	42%	12%	45%
% of Boring	34.6	8.5	21.5
P3-IRM-VAP14 Thickness (ft)	54%	13%	33%
% of Boring	26.5	6.3	18.4
P3-IRIVI-VAP15 Thickness (ft)	52%	12%	36%
% of Boring	34.1	8.8	15.5
P3-IRIVI-VAP16 Thickness (ft)	58%	15%	27%
Nof Boring % of Boring	40.0	11.0	2.9
Thickness (ft)	74%	20%	5%
Nof Boring	29.3	10.8	4.6
Thickness (ft)	65%	24%	10%
% of Boring	28.1	8.3	4.1
Thickness (ft)	69%	21%	10%
100 cc1 <sup>1</sup> % of Boring	21.85	5.15	14.75
Thickness (ft)	52%	12%	35%
% of Boring	28.15	10.65	16.65
Thickness (ft)	51%	19%	30%
Combined Thickness (ft)	692.8	203.4	368.0
% of Boring	54.8%	16.1%	29.1%

Notes:

1

Site 199 borings were advanced concurrent with the Phase III IRM borings so have been combined with the Phase III IRM locations for data review.



FIGURE E1

 $C_3$ 

45

50



- $C1-most\ permeable\ soils,\ dominated\ by\ advective\ transport$
- $\ensuremath{\text{C2}}\xspace$  moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion





C3 – low permeability soils, dominated by diffusion







C3 - low permeability soils, dominated by diffusion

FIGURE E4

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GARFIELD AVENUE GROUP SITES

JERSEY CITY, NEW JERSEY

114-P2-MW-6D

Design & Consultancy for natural and built assets

C3 - low permeability soils, dominated by diffusion

FIGURE E6











FIGURE E10

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- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion







FIGURE E15

**C**<sub>3</sub>

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FIGURE Design & Consultancy for natural and built assets E20











FIGURE E25

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FIGURE E27

C<sub>3</sub>

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114-P2-IRM-89D

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FIGURE E28











- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC (mS/m) HPT Flow Max (mL/min) HPT Press. Max (psi) Corr. HPT Press. (psi) Est. K (ft/day) 100 210 16 40 60 80 90 200 250 300 0 20 40 62 0 50 100 150170 P3 IRM VAP01 C1 68.8 % 0.8 C2 16.7 % C3 14.5 % C1+C2 = 85.5 % K1 to K2 = 31 ft/d K2 to K3 = 19 ft/d  $C_3$ 15 Depth (ft) 20  $C_2$ 25 30 0.2 35  $\mathbf{C}_1$ 0.1 40 43 0.0 80 90 60 16 40 Abs. Piezometric Pressure (psi) 0 4 8 12 16 20 24 28 32 36 Boring Thickness (ft) **PPG INDUSTRIES** GARFIELD AVENUE GROUP SITES JERSEY CITY, NEW JERSEY Notes: C1 - most permeable soils, dominated by advective transport

C2 - moderately permeable soils, dominated by slow advection

C3 - low permeability soils, dominated by diffusion



1.0 0.9 EC (mS/m  $\times 10^3$ ) HPT Press. Max (psi) Est. K (ft/day) HPT Flow Max (mL/min) Corr. HPT Press. (psi) P3 IRM VAP02 0.0 0.5 1.0 10 50 110 150 250 62 100 150170 200 300320 0 20 40 0 50 C1 53.3 % 0.8 C2 21.4 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 25.3 % 2 V C1+C2 = 74.7 % 15 -K1 to K2 = 18 ft/d K2 to K3 = 7.3 ft/d 20 **C**<sub>3</sub> 25  $\mathbf{C}_{2}$ Depth (ft) 30 35 - $\mathbf{C}_1$ 40 45 0.2 50 55 0.1 60 -62 0.0 50 10 110 Abs. Piezometric Pressure (psi) 30 0 10 40 20 50 60 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC  $(mS/m \times 10^3)$ HPT Press. Max (psi) Est. K (ft/day) HPT Flow Max (mL/min) Corr. HPT Press. (psi) P3 IRM VAP03 0.0 0.5 1.0 10 100 120 40 100 300 350 0 50 90 100 150170 50 200 0 50 C1 44.1% 0.8 C2 17.1 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 38.8 % C1+C2 = 61.2 %  $C_3$ 15 -K1 to K2 = 19 ft/d 20 K2 to K3 = 5.4 ft/d  $\mathbf{C}_{\mathbf{2}}$ 25 30 (#) Depth (  $C_1$ 40 -45 -50 0.2 55 -60 -0.1 65 -68 0.0 100 120 50 10 Abs. Piezometric Pressure (psi) 30 0 10 20 40 50 60 Boring Thickness (ft) **PPG INDUSTRIES** 

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 HPT Flow Max (mL/min) Corr. HPT Press. (psi) EC (mS/m) HPT Press. Max (psi) Est. K (ft/day) 200 400 470 10 50 100 120 170 200 300 390 0 50 110 0 50 100 150170 P3 IRM VAP04 C1 58.9 % 0.8 C2 13.6 % C3 27.5 % 15 C1+C2 = 72.5 %  $C_3$ 20 K1 to K2 = 42 ft/d 25 K2 to K3 = 16 ft/d 30 35 40  $\mathbf{C}_2$ Depth (ft) 45 50  $\mathbf{C}_1$ 55 60 65 -70 0.2 75 80 0.1 85 90 0.0 100 120 50 10 Abs. Piezometric Pressure (psi) 0 50 10 20 30 40 60 70 80 Boring Thickness (ft) **PPG INDUSTRIES** 

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC (mS/m) HPT Press. Max (psi) HPT Flow Max (mL/min) Corr. HPT Press. (psi) Est. K (ft/day) 100 200 300 10 50 100 120 180 200 250 280 0 20 40 62 0 50 100 150170 P3 IRM VAP05 C1 56.5 % 0.8 C2 14.0 % C3 29.6 % 10 15 C1+C2 = 70.4 % J. 20 K1 to K2 = 35 ft/d 25 K2 to K3 = 14 ft/d  $\mathbf{C}_{3}$ 30 35 40  $\mathbf{C}_2$ Depth (ft) 45 50 -**C**<sub>1</sub> 55 60 65 70 0.2 3 75 80 0.1 85 90 0.0 100 120 50 10 Abs. Piezometric Pressure (psi) 0 50 10 20 30 40 60 70 80 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC (mS/m  $\times 10^3$ ) HPT Press. Max (psi) Est. K (ft/day) HPT Flow Max (mL/min) Corr. HPT Press. (psi) P3 IRM VAP06 3.8 10 50 110 160 200 300 360 53 0 100 150170 2 3 20 40 50 0 C1 71.6 % 0.8 C2 21.2 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 7.2 % 10 C1+C2 = 92.8 % 15 -K1 to K2 = 27 ft/d 20 K2 to K3 = 11 ft/d 25  $C_2$ 30 Depth (ft) 35 40  $C_1$ 45 50 ş 55 0.2 60 0.1 65 71 -0.0 50 10 110 Abs. Piezometric Pressure (psi) 30 0 10 20 40 50 60 70 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC  $(mS/m \times 10^3)$ HPT Press. Max (psi) Est. K (ft/day) HPT Flow Max (mL/min) Corr. HPT Press. (psi) P3 IRM VAP07 0.5 1.0 10 100 120 160 250 310 0 50 90 100 150170 0.0 50 200 0 50 C1 56.6 % 0.8 C2 15.0 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 28.4 % C1+C2 = 71.6 % 15 K1 to K2 = 41 ft/d 20 K2 to K3 = 14 ft/d  $C_3$ 25 30  $C_2$ Depth (ft) 35 40  $\mathbf{C}_{1}$ 45 50 0.2 55 60 0.1 65 70 0.0 100 120 10 50 Abs. Piezometric Pressure (psi) 0 10 30 40 50 20 60 70 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC  $(mS/m \times 10^3)$ HPT Press. Max (psi) Est. K (ft/day) HPT Flow Max (mL/min) Corr. HPT Press. (psi) P3 IRM VAP08 2.9 10 100 120 150 75 100 150170 2 50 200 250 300320 0 20 40 60 0 50 C1 51.5 % 0.8 C2 12.6 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 35.9 % 10 C1+C2 = 64.1 % 15 K1 to K2 = 44 ft/d 20 K2 to K3 = 11 ft/d 25  $C_3$ 30 35  $\mathbf{C}_{2}$ Depth (ft) 40 -45 - $C_1$ 50 -55 -60 -0.2 65 70 -0.1 75 -79 0.0 50 100 120 10 Abs. Piezometric Pressure (psi) 40 50 0 10 20 30 60 70 80 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC (mS/m  $\times 10^3$ ) HPT Press. Max (psi) Est. K (ft/day) HPT Flow Max (mL/min) Corr. HPT Press. (psi) P3 IRM VAP09 3 3.5 10 100 120 140 76 100 150170 2 50 200 250 300320 0 20 40 60 0 50 C1 41.0 % 0.8 C2 14.5 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 44.5 % 10 -C1+C2 = 55.5 % 15 -K1 to K2 = 44 ft/d 20 -K2 to K3 = 14 ft/d 25  $C_2$ 30 € 35 Depth 40  $\mathbf{C}_1$ 45 50 55 0.2 60 -65 -0.1 70 73 0.0 100 120 50 10 Abs. Piezometric Pressure (psi) 30 40 0 10 20 50 70 60 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC  $(mS/m \times 10^3)$ HPT Press. Max (psi) HPT Flow Max (mL/min) Est. K (ft/day) Corr. HPT Press. (psi) P3 IRM VAP10 10 100 120 100 320 0 20 75 100 150170 2 3 50 200 40 60 0 50 C1 51.8 % 0.8 C2 14.3 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 33.9 % 10 C1+C2 = 66.1 % 15 K1 to K2 = 59 ft/d 20 **C**<sub>3</sub> K2 to K3 = 17 ft/d 25 30  $\mathbf{C}_2$ 35 (#) utdag 40 - $\mathbf{C}_1$ 45 50 55 0.2 60 65 -0.1 70 -76 -0.0 50 100 120 10 Abs. Piezometric Pressure (psi) 30 0 10 20 40 50 60 70 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC  $(mS/m \times 10^3)$ HPT Press. Max (psi) Est. K (ft/day) HPT Flow Max (mL/min) Corr. HPT Press. (psi) P3 IRM VAP11 4.3 10 100 120 170 200 300 320 0 20 40 60 73 0 100 150170 2 50 250 50 C1 47.2 % 0.8 C2 20.3 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 32.5 % C1+C2 = 67.5 % 20 K1 to K2 = 60 ft/d K2 to K3 = 10 ft/d 30 **C**<sub>2</sub> 40 Depth (ft) 50 - $C_1$ 60 -70 -0.2 80 0.1 1 90 0.0 100 120 10 50 Abs. Piezometric Pressure (psi) 0 50 10 20 30 40 60 70 80 90 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 HPT Press. Max (psi) HPT Flow Max (mL/min) EC (mS/m) Corr. HPT Press. (psi) Est. K (ft/day) 200 400 630 10 50 100 120 160 200 250 290 0 20 40 60 73 50 100 150170 P3 IRM VAP12 0 C1 51.6 % 0.8 C2 13.3 % C3 35.1 % 10 -C1+C2 = 64.9 % 20 -K1 to K2 = 33 ft/d K2 to K3 = 7.5 ft/d  $\mathbf{C}_3$ 30 - $\mathbf{C}_2$ 40 -Depth (ft) 50 - $\mathbf{C}_1$ 60 -70 -0.2 80 -90 -0.1 100 -0.0 100 120 50 10 Abs. Piezometric Pressure (psi) 50 0 10 20 40 60 70 30 80 90 Boring Thickness (ft) **PPG INDUSTRIES** GARFIELD AVENUE GROUP SITES JERSEY CITY, NEW JERSEY

## Notes:

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



P3-IRM-VAP12

ARCADIS

1.0 0.9 EC (mS/m  $\times 10^3$ ) Corr. HPT Press. (psi) HPT Press. Max (psi) HPT Flow Max (mL/min) Est. K (ft/day) P3 IRM VAP13 1.1 10 100 120 40 100 200 300 0 50 80 20 40 60 73 0.0 0.5 50 0 C1 42.3 % 0.8 C2 12.3 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 45.4 % 10 C1+C2 = 54.6 % 15 K1 to K2 = 21 ft/d20 K2 to K3 = 6.9 ft/d  $C_3$ 25 -30  $C_2$ 35 Depth (ft) 40 45 - $\mathbf{C}_1$ 50 55 60 0.2 65 -70 -0.1 75 80 0.0 100 120 10 50 Abs. Piezometric Pressure (psi) 10 30 40 0 20 50 60 70 Boring Thickness (ft) **PPG INDUSTRIES** GARFIELD AVENUE GROUP SITES JERSEY CITY, NEW JERSEY

# Notes:

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion

P3-IRM-VAP13

ARCADIS

Design & Consultancy for natural and built assets FIGURE

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1.0 0.9 EC (mS/m) HPT Press. Max (psi) HPT Flow Max (mL/min) Corr. HPT Press. (psi) Est. K (ft/day) 200 400450 10 50 100 120 110 200 310 0 20 40 60 79 50 100 150170 P3 IRM VAP14 0 C1 53.5 % 0.8 C2 13.2 % C3 33.3 % C1+C2 = 66.7 % 15 K1 to K2 = 59 ft/d **C**<sub>3</sub> 20 K2 to K3 = 16 ft/d 25 30 С, (II) John (II) 35 40  $\mathbf{C}_1$ 45 50 55 0.2 60 65 0.1 70 73 0.0 50 100 120 10 Abs. Piezometric Pressure (psi) 30 0 10 20 40 50 70 60 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC (mS/m  $\times 10^3$ ) HPT Press. Max (psi) Est. K (ft/day) HPT Flow Max (mL/min) Corr. HPT Press. (psi) P3 IRM VAP15 2.7 10 100 120 180 200 250 300 320 0 50 90 50 110 1 2 50 0 C1 51.7 % 0.8 C2 12.2 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 36.1 % C1+C2 = 63.9 % 15 K1 to K2 = 39 ft/d K2 to K3 = 16 ft/d**C**<sub>3</sub> 20 25  $\mathbf{C}_2$ Depth 35  $\mathbf{C}_1$ 40 45 0.2 50 0.1 0.0 100 120 10 50 Abs. Piezometric Pressure (psi) 30 0 10 40 20 50 60 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC (mS/m  $\times 10^3$ ) HPT Flow Max (mL/min) Corr. HPT Press. (psi) Est. K (ft/day) HPT Press. Max (psi) P3 IRM VAP16 3.2 10 100 120 120 150 200 250 310 0 50 90 100 150170 2 50 0 50 0 C1 58.3 % 0.8 C2 15.1 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 26.6 % 10 C1+C2 = 73.4 % 7 5 15 K1 to K2 = 32 ft/d K2 to K3 = 17 ft/d 20 **C**<sub>3</sub> 25  $C_2$ 30 (#) Depth (  $\mathbf{C}_{1}$ 40 -45 50 -0.2 55 -60 0.1 65 -67 0.0 100 120 10 50 Abs. Piezometric Pressure (psi) 30 0 10 40 20 50 60 Boring Thickness (ft) **PPG INDUSTRIES** 

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC (mS/m  $\times 10^3$ ) HPT Flow Max (mL/min) Corr. HPT Press. (psi) Est. K (ft/day) HPT Press. Max (psi) P3 IRM VAP17 3.2 10 100 120 120 150 200 250 310 0 50 90 100 150170 2 50 0 50 0 C1 74.2 % 0.8 C2 20.5 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 5.4 % 10 C1+C2 = 94.6 % 7 4 15 K1 to K2 = 35 ft/d K2 to K3 = 15 ft/d 20 25 30  $C_2$ (4) Depth ( 40 -**C**<sub>1</sub> 45 50 -0.2 55 -60 0.1 65 -67 0.0 100 120 10 50 Abs. Piezometric Pressure (psi) 0 10 30 20 40 50 60 Boring Thickness (ft) **PPG INDUSTRIES** GARFIELD AVENUE GROUP SITES

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC (mS/m  $\times 10^3$ ) Est. K (ft/day) HPT Press. Max (psi) HPT Flow Max (mL/min) Corr. HPT Press. (psi) P3 IRM VAP18 0.0 0.1 0.2 10 100 120 190 300 400 20 50 110 50 0 40 60 76 0 C1 65.5 % 0.8 C2 24.2 % Cumulative Distribution of "K" (CDF<sub>k</sub>) 8.0 9.0 2.0 9.0 2.0 C3 10.3 % C1+C2 = 89.7 % 10 K1 to K2 = 20 ft/d 15 K2 to K3 = 10 ft/d**C**<sub>3</sub> 20 25 (#) С, Depth 30 -¥ 35 40 45 -0.2 50 0.1 55 3 58 0.0 100 120 10 50 Abs. Piezometric Pressure (psi) 0 5 25 10 15 20 30 35 40 45 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion



1.0 0.9 EC (mS/m) HPT Press. Max (psi) HPT Flow Max (mL/min) Corr. HPT Press. (psi) Est. K (ft/day) 100 200 250 16 40 60 80 94 160 200 300 360 0 20 40 52 0 50 100 150170 P3 IRM VAP19 0 C1 69.3 % 0.8 C2 20.5 % C3 10.1 % 5 -C1+C2 = 89.9 % 10 -K1 to K2 = 21 ft/dK2 to K3 = 9.1 ft/d15 **C**<sub>2</sub> 20  $\mathbf{C}_2$ (tj) (tj)  $C_1$ 30 35 0.2 40 -0.1 45 -49 0.0 60 80 94 16 40 1 Abs. Piezometric Pressure (psi) 5 25 0 10 15 20 30 35 40 45 Boring Thickness (ft)

- C1 most permeable soils, dominated by advective transport
- C2 moderately permeable soils, dominated by slow advection
- C3 low permeability soils, dominated by diffusion





